

# **Geotechnical Policy**

Kosciuszko Alpine Resorts

# Form 4 – Minimal Impact Certification

DA Number:

This form may be used where minor construction works which present minimal or no geotechnical impact on the site or related land are proposed to be erected within the "G" line area of the geotechnical maps.

A geotechnical engineer or engineering geologist must inspect the site and/or review the proposed development documentation to determine if the proposed development requires a geotechnical report to be prepared to accompany the development application. Where the geotechnical engineer determines that such a report is not required then they must complete this form and attach design recommendations where required. A copy of Form 4 with design recommendation, if required, must be submitted with the development application.

### Please contact the Alpine Resorts Team in Jindabyne for further information - phone 02 6456 1733.

To complete this form, please place a cross in the appropriate boxes  $\Box$  and complete all sections.

1. Declaration made by geotechnical engineer or engineering geologist in relation to a nil or minimal geotechnical impact assessment and site classification

I, Mr⊠	Ms 🗌	Mrs 🗌	Dr 🗌	Other		
First Na	me				Family Name	
Mark					Bartel	

OF

Company/organisation

Asset Geotechnical Engineering Pty Ltd (trading as AssetGeoEnviro)

certify that I am a geotechnical engineer /engineering geologist as defined by the "Policy" and I have inspected the site and reviewed the proposed development known as

Smiggin Holes Relocatable Locker Room

As a result of my site inspection and review of the following documentation

(List of documentation reviewed)

Plans by CLM Civil; project: U-214; drawing numbers: U-214-1, 2, 3; rev: D; dated 29/08/2023

Structural Design by Camstruct Consulting; project: 23023-S01 to S05; rev: A; dated: 08/09/2023

Structural Design Certificate by Camstruct Consulting, ref: 23023 CERT; dated: 08/09/2023

I have determined that;

- ☑ the current load-bearing capacity of the existing building will not be exceeded or adversely impacted by the proposed development, and
- the proposed works are of such a minor nature that the requirement for geotechnical advice in the form of a geotechnical report, prepared in accordance with the "Policy", is considered unnecessary for the adequate and safe design of the structural elements to be incorporated into the new works, and
- in accordance with AS 2870.1 Residential Slabs and Footings, the site is to be classified as a type

(insert classification type)
Class P (low bearing capacity soils present)

■ I have attached design recommendations to be incorporated in the structural design in accordance with this site classification.

I am aware that this declaration shall be used by the Department as an essential component in granting development consent for a structure to be erected within the "G" line area (as identified on the geotechnical maps) of Kosciuszko Alpine Resorts without requiring the submission of a geotechnical report in support of the development application.

# 2. Signatures

Signature	Chartered professional status
Mark Bartil	CPEng 35641 NER (Civil)
WALL DO CO	
Name	Date
Mark Bartel	14 September 2023

# 3. Contact details

### **Alpine Resorts Team**

Shop 5A, 19 Snowy River Avenue P O Box 36, JINDABYNE NSW 2627 Telephone: 02 6456 1733 Facsimile: 02 6456 1736 Email: alpineresorts@planning.nsw.gov.au

### Investigation findings and design recommendations:

Ground surface at building location is saturated with surface water flow. DCP conducted (attached) indicated loose and very loose soils to 0.5m below ground level (bgl), medium dense to about 1.5m, dense to about 2m, and very dense from 2m to termination at 2.5m depth. Screw piles are recommended, and should be founded within assessed very dense soils below nominal 2m bgl, designed for maximum allowable end bearing pressure of 600 kPa.

This advice to be read in conjunction with attached Important Information about your Geotechnical Report and Explanation Sheets.

AssetGeoEnviro accepts no liability where our recommendations are not followed or are only partially followed.



### Scope of Services

The geotechnical report ("the report") has been prepared in accordance with the scope of services as set out in the contract, or as otherwise agreed, between the Client and Asset Geotechnical Engineering Pty Ltd ("Asset"), for the specific site investigated. The scope of work may have been limited by a range of factors such as time, budget, access and/or site disturbance constraints.

The report should not be used if there have been changes to the project, without first consulting with Asset to assess if the report's recommendations are still valid. Asset does not accept responsibility for problems that occur due to project changes if they are not consulted.

### **Reliance on Data**

Asset has relied on data provided by the Client and other individuals and organizations, to prepare the report. Such data may include surveys, analyses, designs, maps, and plans. Asset has not verified the accuracy or completeness of the data except as stated in the report. To the extent that the statements, opinions, facts, information, conclusions and/or recommendations ("conclusions") are based in whole or part on the data, Asset will not be liable in relation to incorrect conclusions should any data, information or condition be incorrect or have been concealed, withheld, misrepresented, or otherwise not fully disclosed to Asset.

### **Geotechnical Engineering**

Geotechnical engineering is based extensively on judgment and opinion. It is far less exact than other engineering disciplines. Geotechnical engineering reports are prepared for a specific client, for a specific project and to meet specific needs, and may not be adequate for other clients or other purposes (e.g., a report prepared for a consulting civil engineer may not be adequate for a construction contractor). The report should not be used for other than its intended purpose without seeking additional geotechnical advice. Also, unless further geotechnical advice is obtained, the report cannot be used where the nature and/or details of the proposed development are changed.

### Limitations of Site Investigation

The investigation program undertaken is a professional estimate of the scope of investigation required to provide a general profile of subsurface conditions. The data derived from the site investigation program and subsequent laboratory testing are extrapolated across the site to form an inferred geological model, and an engineering opinion is rendered about overall subsurface conditions and their likely behavior regarding the proposed development. Despite investigation, the actual conditions at the site might differ from those inferred to exist, since no subsurface exploration program, no matter how comprehensive, can reveal all subsurface details and anomalies.

The engineering logs are the subjective interpretation of subsurface conditions at a particular location and time, made by trained personnel. The actual interface between materials may be more gradual or abrupt than a report indicates.

Therefore, the recommendations in the report can only be regarded as preliminary. Asset should be retained during the project implementation to assess if the report's recommendations are valid and whether changes should be considered as the project proceeds.

### Subsurface Conditions are Time Dependent

Subsurface conditions can be modified by changing natural forces or man-made influences. The report is based on conditions that existed at the time of subsurface exploration. Construction operations adjacent to the site, and natural events such as floods, or ground water fluctuations, may also affect subsurface conditions, and thus the continuing adequacy of a geotechnical report. Asset should be kept appraised of any such events and should be consulted to determine if any additional tests are necessary.

### Verification of Site Conditions

Where ground conditions encountered at the site differ significantly from those anticipated in the report, either due to natural variability of subsurface conditions or construction activities, it is a condition of the report that Asset be notified of any variations and be provided with an opportunity to review the recommendations of this report. Recognition of change of soil and rock conditions requires experience, and it is recommended that a suitably experienced geotechnical engineer be engaged to visit the site with sufficient frequency to detect if conditions have changed significantly.

### **Reproduction of Reports**

This report is the subject of copyright and shall not be reproduced either totally or in part without the express permission of this Company. Where information from the accompanying report is to be included in contract documents or engineering specification for the project, the entire report should be included to minimize the likelihood of misinterpretation from logs.

### **Report for Benefit of Client**

The report has been prepared for the benefit of the Client and no other party. Asset assumes no responsibility and will not be liable to any other person or organisation for or in relation to any matter dealt with or conclusions expressed in the report, or for any loss or damage suffered by any other person or organisation arising from matters dealt with or conclusions expressed in the report (including without limitation matters arising from any negligent act or omission of Asset or for any loss or damage suffered by any other party relying upon the matters dealt with or conclusions expressed in the report). Other parties should not rely upon the report or the accuracy or completeness of any conclusions and should make their own inquiries and obtain independent advice in relation to such matters.

### Data Must Not Be Separated from The Report

The report presents the site assessment and must not be copied in part or altered in any way.

Logs, figures, drawings, test results etc. included in our reports are developed by professionals based on their interpretation of field logs (assembled by field personnel) and laboratory evaluation of field samples. These data should not under any circumstances be redrawn for inclusion in other documents or separated from the report in any way.

### **Report Recommendations not Followed**

Where the recommendations of the report are not followed or are only partially followed, there may be significant implications for the project (e.g., commercial loss, property loss or damage, personal injury, or loss of life). Consult Asset if you are not intending to follow all the report recommendations, to assess what the implications could be. Asset does not accept responsibility where the report recommendations have not been followed or have only been partially followed.

### **Other Limitations**

Asset will not be liable to update or revise the report to consider any events or emergent circumstances or fact occurring or becoming apparent after the date of the report.

# Soil and Rock Explanation Sheets (1 of 2)

NE

HF

ΒH

ΕX DZ

R



# Log Abbreviations & Notes

# METHOD

borehole logs			
AS	auger screw *		
AD	auger drill *		
RR	roller / tricone		
W	washbore		
CT	cable tool		
HA	hand auger		
D	diatube		
В	blade / blank bit		
V	V-bit		
Т	TC-bit		
* bit shown by suffix e.a. ADV			

*	bit	shown	by	suffix	e.g.	ADV
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<u>coring</u> NMLC, NQ, PQ, HQ

### SUPPORT

boreh	ole logs	excavation logs		
Ν	nil	N	nil	
Μ	mud	S	shoring	
С	casing	В	benched	
NO	NO rods			

### CORE-LIFT

		casing installed
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barrel withdrawn  $\vdash$ 

### NOTES, SAMPLES, TESTS

D	disturbed
В	bulk disturbed
U50	thin-walled sample, 50mm diameter
HP	hand penetrometer (kPa)
SV	shear vane test (kPa)
DCP	dynamic cone penetrometer (blows per 100mm penetration)
SPT	standard penetration test
N*	SPT value (blows per 300mm)
	* denotes sample taken
Nc	SPT with solid cone
R	refusal of DCP or SPT

### USCS SYMBOLS

- Gravel and gravel-sand mixtures, little or no fines. GW
- GP Gravel and gravel-sand mixtures, little or no fines, uniform gravels
- GΜ Gravel-silt mixtures and gravel-sand-silt mixtures. GC
- Gravel-clay mixtures and gravel-sand-clay mixtures. Sand and gravel-sand mixtures, little or no fines. SW
- SP Sand and gravel sand mixtures, little or no fines.
- SM Sand-silt mixtures.
- SC Sand-clay mixtures.
- ML Inorganic silt and very fine sand, rock flour, silty or clayey fine sand or silt with low plasticity. Inorganic clays of low to medium plasticity, gravelly clays, sandy

DENSITY INDEX

- CL, CI clays.
- Organic silts OL
- ΜН
- Inorganic silts Inorganic clays of high plasticity. СН Organic clays of medium to high plasticity, organic silt
- ОH ΡT Peat, highly organic soils.

### **MOISTURE CONDITION**

- dry moist D
- Μ W
- wet plastic limit Wp
- wi iquid limit

### CONSISTENCY

VS	very soft	VL	very loose
S	soft	L	loose
F	firm	MD	medium dense
St	stiff	D	dense
VSt	very stiff	VD	very dense
Н	hard		
Fb	friable		

# excavation logs

- natural excavation hand excavation backhoe bucket excavator bucket dozer blade ripper tooth

# Graphic Log





Other

### **Boundaries**

- -- Probable
- ---- Possible

#### WEATHERING Х

WEA	THERING	STR	STRENGTH	
XW	extremely weathered	VL	very low	
HW	highly weathered	L	low	
MW	moderately weathered	М	medium	
SW	slightly weathered	Н	high	
FR	fresh	VH	very high	
		EH	extremely high	

### **RQD (%)**

sum of intact core pieces > 2 x diameter x 100 total length of core run drilled

### **DEFECTS:**

<u>type</u>		<u>c</u>	oating
JT	joint	cl	clean
PT	parting	st	stained
SZ	shear zone	ve	veneer
SM	seam	CO	coating
shape		rough	iness
pl	planar	ро	polished

pl	planar	ро	polished
cu	curved	sl	slickensided
un	undulating	sm	smooth
st	stepped	ro	rough
ir	irregular	vr	very rough

#### inclination

measured above axis and perpendicular to core

# Soil and Rock Explanation Sheets (2 of 2)



<u>Spacing</u> 200 - 600 600 - 2,000

> 2.000

# AS1726-2017

Soils and rock are described in the following terms, which are broadly in accordance with AS1726-2017.

### Soil

### **MOISTURE CONDITION**

#### Description Term

Dry Looks and feels dry. Fine grained and cemented soils are hard, friable or powdery. Uncemented coarse grained soils run freely through hand. Soil feels cool and darkened in colour. Fine grained soils can be moulded. Moist Coarse soils tend to cohere.

Wet As for moist, but with free water forming on hand.

Moisture content of cohesive soils may also be described in relation to plastic limit  $(W_{\text{P}})$  or liquid limit (W\_L) [>> much greater than, > greater than, < less than, << much less than].

### CONSISTENCY OF FINE-GRAINED SOILS

Term	<u>Su (kPa)</u>	Term	<u>Su (kPa)</u>
Very soft	< 12	Very Stiff	>100 - ≤200
Soft	>12 - ≤25	Hard	> 200
Firm	>25 - ≤50	Friable	-
Stiff	>50 - ≤100		

### **RELATIVE DENSITY OF COARSE-GRAINED SOILS**

Term	Density Index (%)	Term	Density Index (%)
Very Loose	< 15	Dense	65 – 85
Loose	15 – 35	Very Dense	>85
Medium Dense	35 – 65		

### PARTICLE SIZE

<u>Name</u>	Subdivision	<u>Size (mm)</u>
Boulders		> 200
Cobbles		63 – 200
Gravel	coarse	19 – 63
	medium	6.7 – 19
	fine	2.36 - 6.7
Sand	coarse	0.6 – 2.36
	medium	0.21 – 0.6
	fine	0.075 – 0.21
Silt		0.002 - 0.075
Clay		< 0.075

### MATERIAL DELINEATION

>65% above 0.075mm Sand or gravel

#### >35% below 0.075mm Clay or silt

### MINOR COMPONENTS

Term	Proportion by Mas	s:
	<u>coarse grained</u>	<u>fine grained</u>
Trace	≤ 5%	≤ 5%
With	>15% ≤ 30%	>5% – ≤12%

#### SOIL ZONING

Layers	Continuous across exposures or sample.
Lenses	Discontinuous, lenticular shaped zones.
Pockets	Irregular shape zones of different material.

### SOIL CEMENTING

Easily broken up by hand pressure in water or air. Weakly Moderately Effort is required to break up by hand in water or in air.

### **USCS SYMBOLS**

Symbol	Description
GW	Gravel and gravel-sand mixtures, little or no fines.
GP	Gravel and gravel-sand mixtures, little or no fines, uniform gravels.
GM	Gravel-silt mixtures and gravel-sand-silt mixtures.
GC	Gravel-clay mixtures and gravel-sand-clay mixtures.
SW	Sand and gravel-sand mixtures, little or no fines.
SP	Sand and gravel sand mixtures, little or no fines.
SM	Sand-silt mixtures.
SC	Sand-clay mixtures.
ML	Inorganic silt and very fine sand, rock flour, silty or clayey fine sand or
	silt with low plasticity.
CL, CI	Inorganic clays of low to medium plasticity, gravelly clays, sandy
	clays.
OL	Organic silts
MH	Inorganic silts
011	In a second second finish all statistics

- Inorganic clays of high plasticity. СН
- Organic clays of medium to high plasticity, organic silt OH
- PT Peat, highly organic soils.

# Rock

### SEDIMENTARY ROCK TYPE DEFINITIONS Ro

Rock Type	Definition (more than 50% of rock consists of)
Conglomerate	gravel sized (>2mm) fragments.
Sandstone	sand sized (0.06 to 2mm) grains.
Siltstone	silt sized (<0.06mm) particles, rock is not laminated.
Claystone	clay, rock is not laminated.
Shale	silt or clay sized particles, rock is laminated.
LAYERING	

### Term

Description No layering apparent. Layering just visible. Little effect on properties. Massive Poorly Developed Well Developed Layering distinct. Rock breaks more easily parallel to layering.

STRUCTURE	
Term	

Term	Spacing (mm)	Term
Thinly laminated	<6	Medium bedded
Laminated	6 – 20	Thickly bedded
Very thinly bedded	20 – 60	Very thickly bedded
Thinly bedded	60 - 200	

STRENGTH	(NOTE: Is50 = Point Loa	d Strength Index)	
Term	<u>ls50 (MPa)</u>	Term	<u>ls50 (MPa)</u>
Very Low	0.03 - 0.1	High	1.0 – 3.0
Low	0.1 – 0.3	Very High	3.0 – 10.0
Medium	0.3 – 1.0	Extremely High	>10.0

## WEATHERING

<u>Term</u>	Description
Residual Soil	Material is weathered to an extent that it has soil properties.
	Rock structures are no longer visible, but the soil has not
	been significantly transported.
Extremely	Material is weathered to the extent that it has soil properties.
	Mass structures, material texture & fabric of original rock is still visible.
Highly	Rock strength is significantly changed by weathering; rock is dis-
	colored, usually by iron staining or bleaching. Some primary min- erals have weathered to clay minerals.
Moderately	Rock strength shows little or no change of strength from fresh rock; rock may be discolored.
Slightly	Rock is partially discolored but shows little or no change of strength from fresh rock.
Fresh	Rock shows no signs of decomposition or staining.

### DEFECT DESCRIPTION

Туре	
Joint	A surface or crack across which the rock has little or no ten- sile strength. May be open or closed.
Parting	A surface or crack across which the rock has little or no ten- sile strength. Parallel or sub-parallel to layering/bedding. May be open or closed.
Sheared Zone	Zone of rock substance with roughly parallel, near planar, curved or undulating boundaries cut by closely spaced joints, sheared surfaces or other defects.
Seam	Seam with deposited soil (infill), extremely weathered insitu rock (XW), or disoriented usually angular fragments of the host rock (crushed).
<u>Shape</u>	
Planar	Consistent orientation.
Curved	Gradual change in orientation.
Undulating	Wavy surface.
Stepped	One or more well defined steps.
Irregular	Many sharp changes in orientation.
Roughness	
Polished	Shiny smooth surface.
Slickensided	Grooved or striated surface, usually polished.
Smooth	Smooth to touch. Few or no surface irregularities.
Rough	Many small surface irregularities (amplitude generally <1mm). Feels like fine to coarse sandpaper.
Very Rough	Many large surface irregularities, amplitude generally >1mm. Feels like very coarse sandpaper.
Coating	
Clean	No visible coating or discolouring.
Stained	No visible coating but surfaces are discolored.
Veneer	A visible coating of soil or mineral, too thin to measure; may be patchy
Coating	Visible coating =1mm thick. Thicker soil material described as seam.